

3D Mapping based-on Integration of UAV Platform and Ground Surveying

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Abstract—Development in aerial photogrammetry technology has contributed a notable impact to the area of large-scale mapping. Nowadays, unmanned aerial vehicle (UAV) platform has become a significant tool in aerial mapping. Generating 3D mapping using photos acquired from UAV is more preferable due to its low cost and flexible operation. Therefore, this study aims to develop a technique for 3D mapping with an integration of UAV aerial photos and detailed ground survey. The produced 3D mapping has RMSE(x) = 0.279, RMSE(y) = 0.215, and RMSE(z) = 1.341 using 25 randomly selected sample points. Besides that, the result shows the location parameters i.e. x, y and z were also positively correlated, t-test(x) = 0.961, t-test(y) = 0.250 and t-test(z) = 1.885, respectively.

Keywords—3D mapping; UAV platform; ground survey; aerial photo

I. INTRODUCTION

The rapid advancement in aerial photogrammetry technology, particularly in the area of digital scanning has produced a new means to create a photorealistic 3D model. 3-dimensional image or also known as 3D image is commonly understood when an image contains the information regarding the depth or height of objects within the image. In realising 3D image production using aerial photos, flight planning parameters and ground control points (GCP) need to be taken into account. GCP is used for georeferencing, which is linking photos with spatial locations. These photos need to further undergo stitching process in order to produce seamless aerial photos of an area, known as aerial orthophoto. The orthophoto can be further processed to generate digital surface model (DSM) and digital terrain model (DTM) in which to be used in producing the 3D mapping. A number of studies related to 3D mapping based on aerial can be found however, accuracy is still an issue. There are effort to look into this issue by researchers in manufacturing-based countries such as USA, China, Japan and UK. Nevertheless, not much effort have been carried out in developing countries such as Malaysia despite having different condition in terms of climate, terrains and land covers in which may have effects on the accuracy. Therefore, this study attempts to further look into the accuracy issue by making use of an approach where aerial photos and ground details are integrated in producing 3D maps.

Nowadays, 3D city mapping can be easily implemented with the advancement of low cost unmanned aerial vehicle (UAV). A 3D city mapping represents an urban environment with three-dimensional geometry of structures and urban objects, with the building as the most prominent features [1-4]. A typical 3D city mapping is derived from various data acquisition techniques, for instance, photogrammetry and laser scanning [5-8], extrusion from 2D footprints [9,10], synthetic aperture radar [11-15], architectural models and drawings [16-18], handheld devices [19,20], procedural modelling [21-26], and volunteered geoformation [27-29].

Meanwhile, the acquisition of aerial photos using UAV platform becomes an efficient optional compared to other methods. Here, UAV allows a higher degree of automation in aerial photos collection [30]. Furthermore, UAV offers a low-cost alternative and a real-time application compared to other classical manned aerial photogrammetry [31]. In addition, UAV photos are also have been proved that applicable in generating high-quality 3D view [31-33]. Subsequently, UAV flown at low altitude (20-30 meters) can produce a ground pixel less than 5 x 5 cm which gives a better resolution for low-cost mapping approach [34].

Numerous research studies have been conducted in term of UAV photogrammetry [35-40] for 3D mapping and modelling. For example, Hudzietz and Saripalli [41] have successfully employed the structure from motion (SfM) techniques for the reconstruction of aerial landscapes imagery. Zhiguang Ding and other scholar verified the superiority of UAV for rapid modelling compared to artificial modelling where the aerial photos were ultimately converted into a 3D image of terrain [42]. The researchers proved that UAV has been used as a platform for aerial photo acquisition. This offers effective and accurate large-scale terrain modelling with cost-efficient solution. Therefore, this study aims to produce a 3D map based on the integration of UAV platform and ground detail survey at 725 acres of the main campus in UTeM, Melaka, Malaysia.

II. ISSUES

Unmanned aerial vehicle (UAV) is a flexible aerial photography platform widely used in various applications

such as mapping, agriculture, 3D documentation and more. Due to benefits provided by UAV, there is no doubt that UAV can be used as tools and sources for data collection. However, despite the numerous benefits, UAV photos are exposed to degradation resulted from issues related to imaging stability. Generally, ordinary UAV considered as lightweight platform which highly influenced by environmental conditions, such as wind [43]. During flight, altitude instability can be caused by wind and other weather factors may lead to a large rotation angle for UAV platform. Large rotation angle from UAV will decrease aerial photo quality and results in photo deformation that will affect their overlaps during stitching process. The insufficient feature overlap between photos may lead to the failure of stitched image reconstruction [44]. Based on the mentioned issue, this will greatly affect the accuracy of the end product that are produced for mapping and 3D documentation. Thus, this study attempts to show the accuracy of 3D mapping that was developed using aerial photos from UAV.

III. STUDY AREA

The location of the study area is UTeM main campus as shown in Figure 1, located in Durian Tunggal, Malacca (Latitude, 2.309980, Longitude 102.317672). The campus coverage area approximately 725 acres and situated near to the bustling historical city of Malacca. UTeM campus which consists of 23 main building and 7 of it is faculty building. For this study, the built-up area at the center of UTeM campus was the main focus where the major buildings are chancellery building, student activity center, mosque, and main hall respectively. The tallest building that recorded for this study is the main hall of UTeM which is 20 meters above mean sea level (MSL). UTeM campus can be easily accessed through Ayer Keroh Toll exit and approximately 20 minutes away from the main city.



Fig. 1. Area of Study (Source Google Earth).

IV. METHODOLOGY

This part describes the workflow for this study. During the early stages of the study, a set of the flowchart is created to make sure the study conducted properly and efficiently. based on Figure 2, the research methodology consists of four (4) main phases which are preliminary study, data acquisition, data processing and results. The first phase is the preliminary study. This phase generally shows the planning process and how the study will be conducted as shown in Figure 2. Preliminary study is essential before conducting any research studies.

In Phase 2, data acquisition focuses mainly the process in obtaining the required data for the study which is ground survey and aerial photos using UAV platform. In ground survey, first, the establishment of ground control point will be conducted followed by detailed survey as the control point will provide referral coordinates for the detailed survey. The flight planning for UAV is created using photogrammetry system and camera calibration for the non-matrix camera is carried out in order to reduce the bundle of error for the orthophoto generation.

Phase 3 describes the process of data processing for ground survey data and aerial photo in order to obtain DSM, DTM, 3D vector and orthophoto respectively. Lastly, Phase 4 is the result and analysis. The result of the study is a 3D orthophoto and 3D vector map of the study area. For accuracy assessment, calculation of RMSE and t-test are implemented.

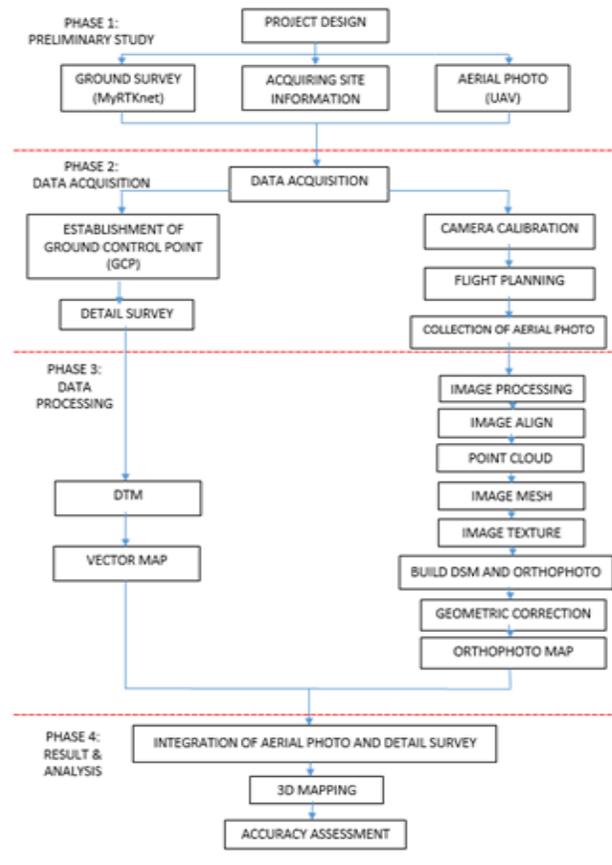


Fig. 2. Flowchart of Study.

A. Data Acquisition

The process of data collection is conducted based on the project frameworks that were collected in the preliminary study. This is to ensure the collected data met the scope of this study. Data collection involves two main parts; primary data and secondary data. For primary data, it involves the collection of aerial photo using non-matrix camera payload UAV platform. Whereas secondary data is ground survey is using GPS and total station.

The data acquisition begins with the establishment of ground control point GCP'S followed by detail survey. The establishment of GCP is done by using GPS Trimble GR-5 at the selected control station. The collection of detailed ground survey data consists several survey tools such as reflectorless total station, handheld GPS and Topcon GPS GR5 respectively in order to obtain the coordinates and elevation (x, y, z) of the feature point.

For the aerial photo, the flight planning of the UAV is conducted and installed in the laboratory by using open source software. In addition, camera calibration has also been conducted to photo distortion. UAV XR Q30 Pro is used for the whole process for collecting the photos.

B. Establishment of Ground Control Point

The establishment of GCPs is a very important stage in the photogrammetric mapping [45]. In this study, ten (10) ground control points have been established. In order to complete the observation of every ground control point, GPS static observation method is applied within 30 minutes' observation for every station. This method is suitable to establish control point at the wide area with sub-centimeter accuracy. GPS observation for control point shall be carried out radially. The concept is known to coordinate at base station and it has been used to compute carrier phase correction in observation. The data observation will be processed using Trimble Total Control software and all coordinates subsequently transformed from WGS84 to local coordinate GDM2000 RSO as shown in Table 1.

TABLE I. COORDINATES OF GCP'S

Station	WGS84		GDM2000 RSO	
	Longitude	Latitude	Longitude	Latitude
1	102.323	2.312	480602.317	255768.550
2	102.323	2.311	480600.904	255718.090
3	102.320	2.309	480347.303	255457.218
4	102.320	2.309	480351.282	255509.274
5	102.318	2.310	480122.095	255595.380
6	102.318	2.310	480127.291	255585.059
7	102.319	2.311	480184.663	255663.234
8	102.319	2.311	480244.406	255653.314
9	102.319	2.312	480176.377	255818.904
10	102.319	2.312	480154.621	255767.354

C. Primary Data

Primary data involves the acquisition of aerial photo using Unmanned Aerial Vehicle (UAV). UAV refers to an aircraft that fly without an onboard human pilot. UAV can be remotely controlled from the ground station or flown autonomously based on the pre-programmed flight planning that installed before the UAV's flown. For this study, a model of UAV XR Q350 Pro that weight 3 kg is used as shown in Figure 3 and the specification of the UAV is shown in Table 2. Furthermore, UAV can maintain a flight time for 25 minutes. UAV is mounted with non-matrix camera model Canon Powershot XS260 with focal length 4.5-90.0 mm as shown in Figure 4 and the specifications are mentioned in Table 3.

The UAV maneuvers at altitude average of 100 m above ground in this study. Each photo have 50% overlap, 65% side lap and a total of 7228 photos were processed using the photogrammetric system. Figure 5 shows a sample aerial photo. The photos then underwent stitching process in order to produce the orthophoto of the study area. In addition, the digital surface model (DSM) and a digital terrain model (DTM) were also produced. The whole process is conducted using i7 processor with 16 GB RAM.



Fig. 3. XR Q30 Pro.

TABLE II. SPECIFICATIONS OF XR Q350 PRO UAV

Criteria	Specification
Main Rotor Diameter	556mm
Main Rotor Blade Length	206mm
Length	289mm
Width	289mm
Height	200mm
Brushless Motor	WK-WS-28-008A
Brushless ESC	WST-15A(G/R)
Receiver	RX703
Flight Time	25 minutes



Fig. 4. Canon Powershot XS260 Camera.

TABLE III. SPECIFICATIONS OF CANON POWERSHOT XS260 CAMERA

Criteria	Lens
Focal Length	4.5 – 90.0 mm (35 mm equivalent: 25 – 500 mm)
Zoom	Optical 20x Zoom Plus 39x Digital Approx. 4.0x (with Digital Tele-Converter Approx. 1.5x or 2.0x and Safety Zoom ¹). Combined Approx. 80x
Maximum f/number	f/3.5 – f/6.8
Construction	12 elements in 10 groups (1 UA lens, 2 double-sided a spherical lens)
Image Stabilization	Yes (lens shift-type), 4-stop. Intelligent IS
Effective Pixels	Approx. 12.1M



Fig. 5. Aerial Photo of the Study Area.

D. Secondary Data

Secondary data involves the data acquisition of ground survey. In this study, the RTK GPS’s surveying technique is used for detailed ground survey. Real-Time Kinematic (RTK) technique is used to improve the precision of the position data derived from satellite-based positioning system. In addition, RTK techniques enable the study to obtain centimeter-level positioning which considered relevant for this study. The system used in this study is Malaysian Real-Time Kinematic GNSS Network (MyRTKnet). MyRTKnet is a system based on a network of seventy-eight (78) global navigation satellite system (GNSS) reference station continuously connected via internet protocol virtual private network (IPVPN) to the control center. At the control center, the computer processor will gather the information from all GPS receiver and creates a living database of Regional Area Connection.

The collections of detailed data were such as spot height, road junctions and other types of data utility. The detailed survey will be processed using CDS (civil design and survey) and AutoCAD. This system was employed to digitize and join the detailed survey data which is obtained using GPS devices and handheld GPS as shown in Figure 6 and Figure 7, respectively. The GPS devices used for this study is Topcon GPS GR-5 and the specification is mentioned in Table 4. Whereas, reflectorless total station shown in Figure 8 will be used to collect the detailed survey of UTeM such as building heights and building edges. The specification of the reflectorless total station as stated in Table 5. Topography map for this study is produced based on the compilation of MyRTKnet and reflectorless total station data. While 3D vector data have been generated from this ground survey.



Fig. 6. Topcon GPS GR-5.



Fig. 7. Handheld GPS.

TABLE IV. TOPCON GPS GR-5 SPECIFICATION

GNSS	
Signals Tracked	GPS: L1, L1C, L2, L2C, and L5 GLONASS: L1, L2 Galileo*: E1, E5a, E5b, AltBOC BeiDou: B1, B2 SBAS L1 C/A WAAS/MSAS/EGNOS QZSS L1 C/A, L1C, L2C
Number of Channels	226-Channel Vanguard Technology with Universal Tracking Channels capable of All-in-View tracking
Antenna Type	Integrated Fence Antenna (1) with Ground Plane
Accuracy	
Static	H: 3mm + 0.1ppm (2) V: 3.5mm + 0.4ppm (2)
RTK	H: 5mm + 0.5ppm V: 10mm + 0.8ppm



Fig. 8. Topcon Total Station FS 105.

TABLE V. TOPCON TOTAL STATION FS 105 SPECIFICATION

Specification	
Display	Dual backlit LCD (ES-107 Single Display)
Battery Operation	Up to 36 hours
Wireless Connection	Bluetooth® Class 1
Operating Temperature	-20°C to +60°C

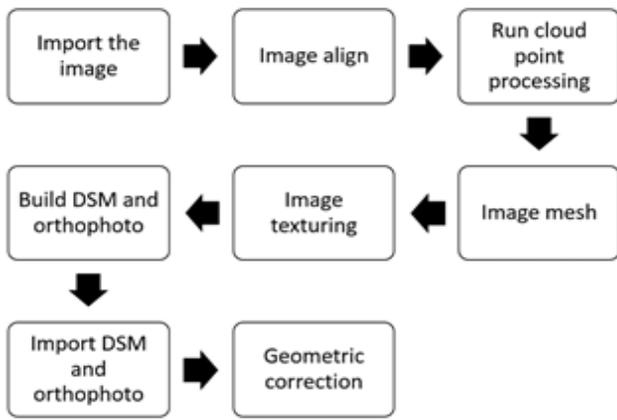


Fig. 9. Flowchart of Image Processing.

E. Data Processing

The aerial photo from UAV will undergo several processes in order to produce the orthophoto of the study area. The first step in producing orthophoto is photo alignment which shows the position and orientation of the photos. In addition, the sparse cloud that produced in this step will reveal the whole area of the study. After the photo alignment, the reconstruction of dense point cloud will be done. It enables the orthophoto that produces in higher detail. Furthermore, the photo alignment is depending on digital matching technique and space intersection using the system.

The following steps are used in photo meshing and texturing. Photo meshing is a process to turn discrete point cloud data into continuous 3D surface and it is constructed in the form of triangulated irregular network (TIN) [46]. In addition, texturing is a process of projecting the texture from the original photo to the model surface [46]. Finally, orthophoto and digital surface model (DSM) were produced. The whole process is shown in Figure 9. The produced orthophoto will undergo geometric corrections in order to transform the coordinates from WGS84 to local coordinate (GDM2000 RSO).

The ground survey data will be processed using Trimble Total Center (TTC) system. Whereby a detailed survey will be processed by using CDS (civil design & survey) and AutoCAD, respectively. The raw data that obtained from the reflectorless total station will be imported to CDS in order to generate the contour line and ground features details. After that, import the data into AutoCAD to join the details. Through this process vector map of the study area will be produced.

F. Generating 3D from Aerial Photo and Detail Survey

The generation of 3D from aerial photo involves two main components which are orthophoto and Digital Surface Model (DSM). The produced orthophoto will provide the location of the object in term of x and y positioning. Subsequently, the digital surface model (DSM) will provide the z values which is the height of an object. By using the module that existed in a GIS system, the DSM value will be referred as the base height and been applied to the orthophoto (2D) in order to produce a 3D model of an object as shown in the results' section.

The process is repeated in order to produce a 3D vector map. While compared to aerial photo the vector drawing that produces from AutoCAD contain the values of z. Thus using the module in the GIS system, a 3D vector map can be produced from 2D vector map.

V. RESULTS

Based on the obtained aerial photo, the orthophoto and DSM for the study area can be produced. Hence, these data can be used to produce a 3D model of the study area. For the obtained featured points through ground detailed survey, it can be used to produce a 3D vector of the study area.

A. 3D Modelling and Orthophoto

Through the data processing, vector map and orthophoto of UTeM main campus are shown in Figure 10 and Figure 11, respectively. In addition, using CDS and AutoCAD system, 3D vector map can be generated as shown in Figure 12. The generated of DSM from the aerial photo will be used to provide height value for orthophoto in order to produce a 3D map based on aerial photo for UTeM main campus (refer to Figure 13). Whereas, Figure 14, Figure 15 and Figure 16 shows the images and 3D visualization of the selected buildings.

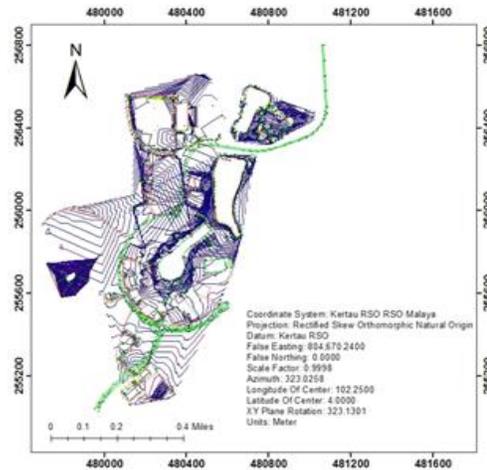


Fig. 10. Vector Map of UTeM Main Campus.

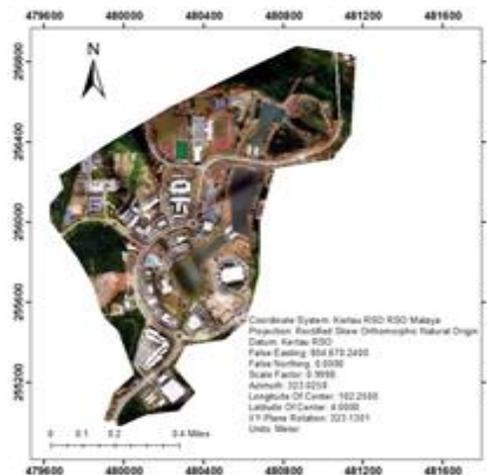


Fig. 11. Orthophoto of UTeM Main Campus.

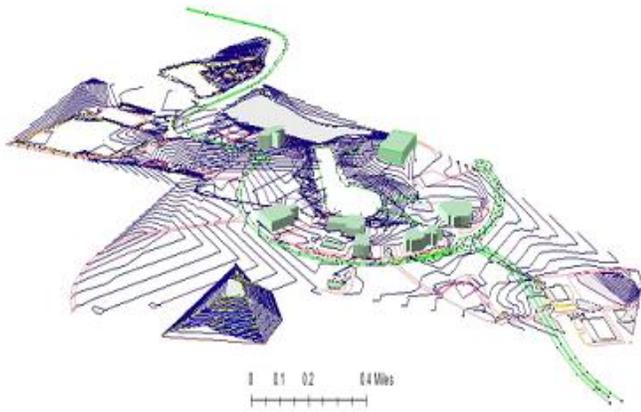


Fig. 12. 3D Vector Map of UTeM Main Campus.



(2D)



Fig. 13. 3D Visualization of the Study Area.



(3D)

Fig. 15. 2D and 3D Map for Mosque.



(2D)



(3D)

Fig. 14. 2D and 3D Map of the Main Hall.



(2D)



(3D)

Fig. 16. 2D and 3D Map for Chancellery Hall.

VI. ANALYSIS

In this section, the quantitative analysis was carried out to show the accuracy for this study. Quantitative analysis is a technique that used to understand the behaviour of the data through mathematical or statistical modelling. Quantitative assessment is implemented by calculating the root mean square error (RMSE) from two different data sources. This study will show the quantitative assessment in term of location or positions which are (x and y) and height (z) from detailed ground survey and raster images from UAV respectively.

$$RMSE = \sqrt{\frac{\sum(n1 - n2)^2}{N}} \quad (1)$$

Where

n1-n2= difference between two parameters

N = total number of points

TABLE VI. TWENTY-FIVE (25) SAMPLE POINTS FROM VECTOR MAP

Point	Vector(x)	Vector(y)	Vector(z)
1	480,197.70	255,668.09	57.53
2	480,221.08	255,676.68	57.53
3	480,195.39	255,635.18	57.53
4	480,210.32	255,635.77	57.53
5	480,244.16	255,653.55	57.53
6	480,117.81	255,605.85	67.08
7	480,133.18	255,575.90	67.08
8	480,168.58	255,597.98	67.08
9	480,156.69	255,621.80	67.08
10	480,334.94	255,458.03	66.66
11	480,314.16	255,482.27	66.66
12	480,392.47	255,487.70	66.66
13	480,351.39	255,534.90	66.66
14	480,608.37	255,726.83	56.98
15	480,602.09	255,769.41	56.98
16	480,611.92	255,769.64	56.98
17	480,151.53	255,546.07	56.46
18	480,169.13	255,554.52	56.46
19	480,031.48	255,570.24	58.84
20	480,010.63	255,573.74	58.84
21	480,005.62	255,589.43	58.84
22	480,021.15	255,601.94	58.84
23	480,454.55	255,997.24	69.87
24	480,399.01	256,037.18	69.87
25	480,396.22	256,031.12	69.87

TABLE VII. TWENTY-FIVE (25) SAMPLE POINTS FROM ORTHOPHOTO

Point	Orthophoto(x)	Orthophoto(y)	Orthophoto(z)
1	480,197.37	255,668.20	55.05
2	480,221.33	255,676.90	55.05
3	480,195.26	255,634.92	55.05
4	480,210.19	255,635.65	55.05
5	480,244.34	255,653.46	55.05
6	480,117.83	255,605.90	67.81
7	480,133.15	255,575.81	67.81
8	480,168.69	255,597.84	67.81
9	480,156.71	255,621.92	67.81
10	480,334.96	255,458.21	66.99
11	480,314.27	255,482.29	66.99
12	480,392.53	255,487.95	66.99
13	480,351.48	255,535.02	66.99
14	480,608.41	255,726.50	55.07
15	480,602.29	255,768.68	55.07
16	480,611.88	255,769.96	55.07
17	480,151.56	255,546.07	56.52
18	480,169.25	255,554.55	56.52
19	480,031.41	255,570.13	58.92
20	480,010.62	255,573.81	58.92
21	480,005.55	255,589.44	58.92
22	480,021.16	255,601.98	58.92
23	480,454.53	255,997.49	70.34
24	480,398.34	256,037.14	70.34
25	480,395.15	256,030.97	70.34

Twenty-five (25) points that randomly selected distributed around the study area were used as the sample point for RMSE calculation (refer to Table 6 and Table 7). By using equation (1) this study managed to obtain RMSE value for coordinates (x and y) and height (z) which are RMSE(x) = 0.279, RMSE(y) = 0.215, and RMSE(z) = 1.341, respectively. This study also conducts pair sample t-test in order to produce a concrete result. Paired sample t-test is a statistical procedure that used to determine whether the mean differences for two set of selected observation is zero. The parameters for paired sample t-test are coordinates (x and y) and height (z) that obtain from 3D raster (refer to Figure 13) and 3D vector (refer to Figure 12). Table 8 and Table 9, respectively show the results for the pair sample correlation and pair sample test respectively.

TABLE VIII. PAIR SAMPLE CORRELATIONS

	N	Correlation
X_vector & X_raster	25	1.000
Y_vector & Y_raster	25	1.000
Z_vector & Z_raster	25	0.990

TABLE IX. PAIR SAMPLE TEST

	Mean	95% Confidence Interval of the Difference		t
		Lower	Upper	
X_vector - X_raster	0.053	-0.061	0.168	0.961
Y_vector - Y_raster	0.010	-0.079	0.101	0.250
Z_vector - Z_raster	0.481	-0.045	1.008	1.885

Based on Table 8 above, the result shows that the parameter obtained from the 3D vector and 3D raster for coordinates (x and y) and height (z) were positively correlated which are $x = 1.000$, $y = 1.000$ and $z = 0.990$ respectively. While Table 9 illustrated the pair sample test, indicates x-coordinates for the 3D vector are 0.053, higher than 3D raster with 95% confidence interval [-0.061, 0.168]. Meanwhile, the test shows y-coordinates for 3D vector is 0.010 higher than 3D raster with 95% confidence interval [-0.079, 0.101]. Lastly, the height (z) from 3D vector was 0.4815 higher than 3D raster with 95% confidence interval [-0.045, 1.008].

VII. CONCLUSION

This paper has presented the use of the unmanned aerial vehicle (UAV) in generating 3D mapping for UTeM, Malacca. This study shows that aerial UAV photos can be used to generate 3D models of features within the selected study area. Even though the generated 3D models do not look alike in the real world, but it manages to outline the shape of the features, for example, the chancellor hall and mosque with good planimetric accuracy rates. However, this study concluded that the accuracy can be improved especially for the height (z) by changing the aerial flight type to oblique photograph instead of vertical photograph or nadir as used in this study (refer to Figure 17).

The implementation of the UAV platform in this study can be described as a low-cost method or approach using close-range photogrammetry in generating large-scale of 3D map compared to other type of conventional methods. Thus, the integration of UAV platform aerial photo and ground detailed survey can be used to produce a large-scale of 3D mapping.

In addition, the result of this study is well supported using accuracy assessments which are RMSE and paired sample t-test. The obtained RMSE values for x, y and z from 25 distributed points are $RMSE_x = 0.279$, $RMSE_y = 0.215$ and $RMSE_z = 1.341$ respectively. For paired sample t-test, the coordinates (x and y) and height (z) from both data were positively correlated.

Furthermore, the study also managed to fulfill the criteria to produce a 3D city model. According to cityGML (Architectural models) there are five (5) types of Level of Detail that used to facilitate efficient visualization and data analysis which are Level of Detail 0 (LOD0), Level of Detail 1 (LOD1), Level of Detail 2 (LOD2), Level of Detail 3 (LOD3) and Level of Detail 4 (LOD4). The positional and height accuracy for each Level of Detail is LOD0 (less than LOD1), LOD1 (5m or less), LOD2 (2m or better), LOD3 (0.5m) and LOD4 (0.2m or less) [47].

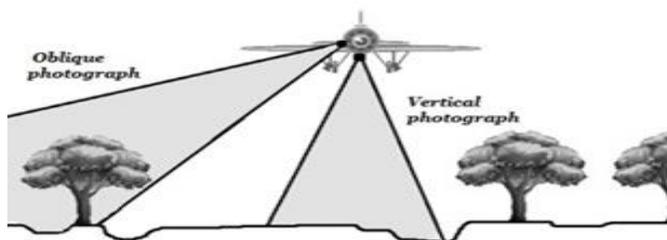


Fig. 17. Type of Aerial Photography.

Referring to cityGML (architectural models), this study has to fulfill the criteria of LOD2 successfully in order to show the positional and height accuracy at 2m or better. Subsequently, the building has differentiated roof structures and thematically differentiated boundary surfaces (refer to Figure 13) which is the criteria of LOD2 [47]. Last but not least, this study managed to show that by using the integration of UAV and detailed surveying method, this study able to produce a 3D campus map for UTeM, Malacca.

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